



UNIVERSITY OF AGDER

FACULTY OF SCIENCE AND TECHNOLOGY

**E X A M**

**Course code: BYG-504**

**Course name: Soil mechanics and foundation**

Date: December 20, 2016

Duration: 9:00-13:00

Number of pages incl. front page: Three pages

Resources allowed: Soil mechanics and foundations by Muni Budhu

Lecture Presentations

One sheet (A4: both sides) of backups (formulae etc. )

Notes: Office kits: compass, calculator etc.

**BYG504 -- Autumn 2016**

1. A soil sample has a moist /bulk unit weight  $\gamma = 20 \text{ kN/m}^3$  at a water content  $w = 10\%$ . Assume the specific gravity of soil solids  $G_s = 2.7$ , try to:
  - a.) determine the void ratio  $e$ ;
  - b.) show its degree of saturation  $S = wG_s/e$ , and calculate it.

*10 points*

2. For the seepage situations as shown in Figure 1, given the saturated specific weight of the soil is  $20 \text{ kN/m}^3$ ;  $g = 10 \text{ m/s}^2$ , try:
  - a.) to calculate the total stress, pore pressure and effective stress at levels of soil on planes as shown X-X in each case (1) and (2).
  - b.) to change the levels of water tank until the effective stress at level X-X is approaching to 0, and specify the total and pore pressure ( you can just do one case from either).

*15 points*

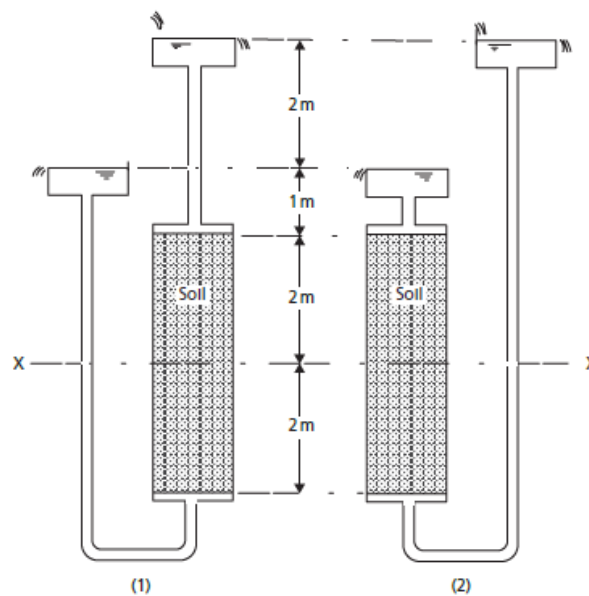


Figure 1

3. An oedometer test on a saturated clay soil gave the following results:  $C_c = 0.2$ ,  $C_r = 0.04$ ,  $\text{OCR} = 4$ ; its unit weight  $\gamma = 20 \text{ kN/m}^3$ . The existing vertical effective stress in the field is  $100 \text{ kPa}$ . A building foundation will increase the vertical stress at the centre of the clay by  $300 \text{ kPa}$ . Assume thickness of the clay layer is  $2 \text{ m}$ .
  - a) Calculate the primary consolidation settlement.
  - b) What would be the difference in settlement if  $\text{OCR}$  was  $1.5$  instead of  $4$ .

*15 points*

4. The initial effective stresses on a saturated soil element at a certain depth in a soil mass are  $\sigma'_1 = 40$  kPa,  $\sigma'_2 = 20$  kPa, and  $\sigma'_3 = 20$  kPa. The groundwater level is well below the soil element. The changes in stresses on the soil element are shown in Fig. 1.

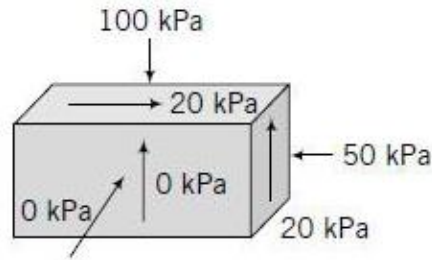


Fig.1

- Calculate the change in principal stresses.
- Plot the total stress path in  $(p, q)$  space, and the effective stress path (assuming that the soil is a linearly elastic material).
- The soil is measured with a strength of  $30^\circ$  in shear, if only the vertical load is increasing, try to predict the maximum of the vertical loading before its failure (with an aid of Mohr-Circle), and to identify the failure direction.

30 points

5. The water content of a sample of saturated soil at a mean effective stress of 20 kPa is 85%. Under a standard CD test, the sample was iso-tropically consolidated with a mean effective stress of 200 kPa. At the end of the consolidation the water content was 65%. The sample was then iso-tropically unloaded to a mean effective stress of 120 kPa; its water content increased by 1%. Calculate  $\lambda$  and  $\kappa$  and to show:  $M_c = 6 \sin \phi'_{cs} / (3 - \sin \phi'_{cs})$ , where  $\phi'_{cs}$  is the critical internal friction angle. Assumed as  $\phi'_{cs} = 30^\circ$ , then:

- To Draw the initial yield surface and the critical state line in  $(p', q)$ ,  $(p', e)$  spaces;
- if a standard CD test is continued by keeping the cell pressure constant while increasing the axial stress. Predict stresses  $p_y$  and  $q_y$ , at its initial yielding, and its stresses  $p_f$  and  $q_f$  at its critical state;
- If a CU test is carried out instead, predicting stresses  $p_y$ ,  $q_y$  and the excessive water pore-pressure at this initial yielding; the stresses  $p_f$ ,  $q_f$  and excessive pore pressure at its critical state.

30 points